89009 REFERENCE

P10. B SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

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<u>LINE</u> **STATION** 12+70.00 TO 17+30.00

PROFILE <u>PLAN</u> N/A

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY _UNION

PROJECT DESCRIPTION REPLACE BRIDGE NO. 92 OVER BEAVERDAM CREEK ON SR 1903

(GILBOA ROAD)

INVENTORY

STATE PROJECT REFERENCE NO. SF-890092

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IM-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NINCLATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

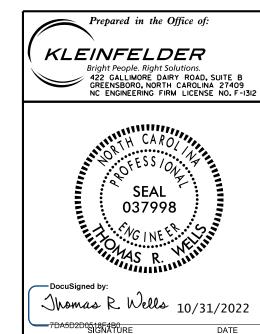
THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESION DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESION INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS, AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

J. KARDON T. WELLS INVESTIGATED BY J. KARDON DRAWN BY T. WELLS CHECKED BY X. BARRETT SUBMITTED BY KLEINFELDER, INC. DATE OCTOBER 2022



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT REPERENCE NO. SHEET NO.

SF-890092

2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

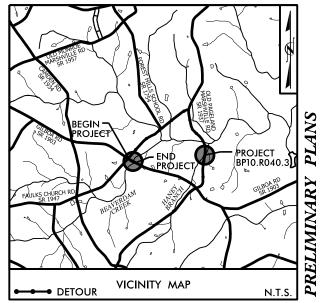
SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DE	SCRIPTION	GRADATION			ROCK DESCRIPTION			TERMS AND DEFINITIONS	
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSO	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.			HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED			ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.		
BE PENETRATED WITH A CONTINUOUS FLIGHT POWE ACCORDING TO THE STANDARD PENETRATION TEST	UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.			ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60			AQUIFER - A WATER BEARING FORMATION OR STRATA.		
IS BASED ON THE AASHTO SYSTEM, BASIC DE	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.			BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.			ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.		
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO (AS MINERALOGICAL COMPOSITION, ANGULARI	ANGULARITY OF GRAINS			ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:			ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING		
VERY STIFF.GRAY,SILTY CLAY,MOIST WITH INTER	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.			WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >			A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.		
SOIL LEGEND AND A	ASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION			ROCK (WR) 100 BLOWS PER FOOT IF TESTED.			ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT	
GENERAL GRANULAR MATERIALS	SILT-CLAY MATERIALS ORGANIC MATERIALS	MINERAL MAN			CRYSTALLINE		E GRAIN IGNEOUS AND METAMOR		WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CLASS. (≤ 35% PASSING *200)	(> 35% PASSING *200)		ES SUCH AS QUARTZ, FELDSPAR, MICA, T DESCRIPTIONS WHEN THEY ARE CONSID		ROCK (CR)	GNEISS, GABBRO	SPT REFUSAL IF TESTED. ROCK , SCHIST, ETC.	TYPE INCLUDES GRANITE,	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 CLASS. A-1-8 A-1-6 A-2-4 A-2-5 A-2-6 A-2-7	A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 A-6, A-7	COMPRESSIBILITY			NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED.			COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM	
0000000000	A-7:6	St ICH.	TLY COMPRESSIBLE	LL < 31	ROCK (NCR)	SEDIMENTARY R	CUDES PHYLLITE, SLATE, SANDST		OF SLOPE.
SYMBOL 000000000000000000000000000000000000		MODER MODER	ATELY COMPRESSIBLE	LL = 31 - 50	COASTAL PLAIN	COASTAL PLAIN	I SEDIMENTS CEMENTED INTO RO	CK. BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
% PASSING	SILT-	HIGHL	Y COMPRESSIBLE	LL > 50	SEDIMENTARY ROI	SPT REFUSAL.	ROCK TYPE INCLUDES LIMESTON TC.	E, SANDSTONE, CEMENTED	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX *40 30 MX 50 MX 51 MN	GHANULAR CLAY MULK,		PERCENTAGE OF MATER	IAL	_		ATHERING		DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX		ORGANIC MATERIAL	GRANULAR SILT - CLAY <u>SOILS</u> <u>SOILS</u>	OTHER MATERIAL	FRESH ROO	K FRESH, CRYSTALS BRIGHT, FEW J	OINTS MAY SHOW SI IGHT STAININ	G. ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL		TRACE OF ORGANIC MA	TTER 2 - 3% 3 - 5%	TRACE 1 - 10%		MER IF CRYSTALLINE.			<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING #40	SOILS WITH	LITTLE ORGANIC MATTI MODERATELY ORGANIC	ER 3 - 5% 5 - 12% 5 - 10% 12 - 20%	LITTLE 10 - 20% SOME 20 - 35%		K GENERALLY FRESH, JOINTS STAIN			DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
LL	40 MX 41 MN 40 MX 41 MN LITTLE OR HIGHLY	HIGHLY ORGANIC	> 10% > 20%	HIGHLY 35% AND ABOVE		STALS ON A BROKEN SPECIMEN FAC A CRYSTALLINE NATURE.	CE SHINE BRIGHTLY. ROCK RINGS	UNDER HAMMER BLOWS IF	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX	8 MX 12 MX 16 MX NO MX AMOUNTS OF ORGANIC		GROUND WATER				IED AND DISCOLODATION EXTENDS	INTO POCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USUAL TYPES STONE FRAGS.	ORGANIC SOILS			TELV ACTED ODILLING		K GENERALLY FRESH.JOINTS STAIN ICH.OPEN JOINTS MAY CONTAIN CL			SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAVEL AND FINE SILTY OR CLAYEY	SILTY CLAYEY MATTER		WATER LEVEL IN BORE HOLE IMMEDIA		CRY	STALS ARE DULL AND DISCOLORED.	. CRYSTALLINE ROCKS RING UNDER	HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND SAND	SOILS SOILS	▼	STATIC WATER LEVEL AFTER 24	HOURS		NIFICANT PORTIONS OF ROCK SHOW			FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING EXCELLENT TO GOOD	FAIR TO POOR FAIR TO POOR UNSUITABLE	√PW	PERCHED WATER, SATURATED ZONE, OR	WATER BEARING STRATA		NITOID ROCKS, MOST FELDSPARS AR L SOUND UNDER HAMMER BLOWS AN			PARENT MATERIAL.
AS SUBGRADE	POOR POOR ORSOTTABLE	O-W-	SPRING OR SEEP			H FRESH ROCK.	to shows storm team 2005 or s	THEROTH AS COM ARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
	30 ; PI OF A-7-6 SUBGROUP IS > LL - 30	0 00			MODERATELY ALL	ROCK EXCEPT QUARTZ DISCOLORED	D OR STAINED. IN GRANITOID ROCK	S, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
CONSISTENCY	MISCELLANEOUS SYMBOLS			SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH					
DOMARY COLL TYPE COMPACTNESS OR	RANGE OF STANDARD RANGE OF UNCONFINED	III BOADWAY EMBA	NEMENT (RE) 25/025 DIP & DIP DIR	ECTION		TESTED, WOULD YIELD SPT REFUSAL		SUUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
CONSISTENCY			ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION OF ROCK STRUCTURES			SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT			LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
VERY LOOSE	< 4	1 4	SPT ST. TEST DOS	SLOPE INDICATOR	(SEV.) RED	DUCED IN STRENGTH TO STRONG SOI	IL. IN GRANITOID ROCKS ALL FELI	SPARS ARE KAOLINIZED	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GENERALLY LOOSE GRANULAR	4 TO 10	SOIL SYMBOL	OPT DMT TEST BOF	INSTALLATION		SOME EXTENT. SOME FRAGMENTS OF TESTED, WOULD YIELD SPT N VALUE			MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS
MATERIAL MEDIUM DENSE	10 TO 30 N/A 30 TO 50	ARTIFICIAL FIL		CONE PENETROMETER TEST	I -			ENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50		THAN ROADWAY EMBANKMENT TEST			VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK			PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE	
VERY SOFT	< 2 < 0.25	— — INFERRED SOIL	. BOUNDARY CORE BORING	 SOUNDING ROD 		MAINING. SAPROLITE IS AN EXAMPLE			OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT	2 TO 4 0.25 TO 0.5		MW NOVE TO THE OWN	TEST BORING		TIGES OF ORIGINAL ROCK FABRIC F			RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY MEDIUM STIFF MATERIAL STIFF	4 TO 8 0.5 TO 1.0 8 TO 15 1 TO 2	INFERRED ROCK	K LINE "" MONITORING WE	WITH CORE		K REDUCED TO SOIL. ROCK FABRIC TTERED CONCENTRATIONS. QUARTZ			ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
(COHESIVE) VERY STIFF	15 TO 3Ø 2 TO 4	***** ALLUVIAL SOIL	BOUNDARY \(\triangle \text{PIEZOMETER} \\ INSTALLATION	SPT N-VALUE		O AN EXAMPLE.	HAT BE TRESENT AS BIKES ON S	INTROCKS. SHI NOCTIC 13	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
HARD > 30 > 4		INSTALLATION			ROCK HARDNESS			SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT	
TEXTURE OR GRAIN SIZE		RECOMMENDATION SYMBOLS			VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES			ROCK.	
U.S. STD. SIEVE SIZE 4 10	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE ACCEPTABLE, BUT NOT TO BE			SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.			SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND		
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053				ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.			RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.	
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY		UNDERCUT	UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK	EMBANKMENT OR BACKFILL					
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)			ABBREVIATIONS					SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.	
GRAIN MM 305 75 2.0			MED MEDIUM	VST - VANE SHEAR TEST				STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF	
SIZE IN. 12 3	0.2 5 0.0 5 0.00 5	AR - AUGER REFUSAL BT - BORING TERMINATED		WEA WEATHERED			HES DEEP BY FIRM PRESSURE OF	KNIFE OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
SOIL MOISTURE - CORRELATION OF TERMS		CL CLAY MOD MODERATELY 7 - UNIT WEIGHT		HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE			WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL		
	CPT - CONE PENETRATION TEST NP - NON PLASTIC 7d - DRY UNIT WEIGHT			POINT OF A GEOLOGIST'S PICK.			TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.		
SOIL MOISTURE SCALE FIELD MOISTURE (ATTERBERG LIMITS) DESCRIPT		CSE COARSE DMT - DILATOMETER TEST	ORG ORGANIC PMT - PRESSUREMETER TE	ST SAMPLE ABBREVIATIONS		I BE GROVED OR GOUGED READILY E IM CHIPS TO SEVERAL INCHES IN S			STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
		DPT - DYNAMIC PENETRAT	ION TEST SAP SAPROLITIC	S - BULK		CES CAN BE BROKEN BY FINGER PR		ICK FUINT. SMHLL, IMIN	
- SATURAT (SAT,)		e - VOID RATIO F - FINE	SD SAND, SANDY	SS - SPLIT SPOON	VERY CAN	BE CARVED WITH KNIFE. CAN BE I	EXCAVATED READILY WITH POINT	OF PICK, PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
LL _ LIQUID LIMIT	THOSE BEEGN THE GROOND WATER TABLE	FOSS FOSSILIFEROUS	SL SILT, SILTY SLI SLIGHTLY	ST - SHELBY TUBE RS - ROCK		MORE IN THICKNESS CAN BE BROKE	EN BY FINGER PRESSURE. CAN BE	SCRATCHED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC	SEMISOLID; REQUIRES DRYING TO	FRAC FRACTURED, FRACT	URES TCR - TRICONE REFUSAL	RT - RECOMPACTED TRIAXIAL		GERNAIL.			TOPSOIL (TS,) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE - WET - (W	ATTAIN OPTIMUM MOISTURE	FRAGS FRAGMENTS HI HIGHLY	w - MOISTURE CONTENT	CBR - CALIFORNIA BEARING RATIO		CTURE SPACING		DING	BENCH MARK: N/A
"" PL L + PLASTIC LIMIT			V - VERY		TERM VERY WIDE	<u>SPACING</u> MORE THAN 10 FEET	TERM VERY THICKLY BEDDED	THICKNESS 4 FEET	
OM _ OPTIMUM MOISTURE - MOIST -	(M) SOLID; AT OR NEAR OPTIMUM MOISTURE		JIPMENT USED ON SUBJECT		WIDE WIDE	3 TO 10 FEET	THICKLY BEDDED	1.5 - 4 FEET	ELEVATION: N/A FEET
SL _ SHRINKAGE LIMIT		DRILL UNITS:	ADVANCING TOOLS:	HAMMER TYPE:	MODERATELY (THINLY BEDDED	0.16 - 1.5 FEET	NOTES:
	REQUIRES ADDITIONAL WATER TO	CME-45C	CLAY BITS	X AUTOMATIC MANUAL	CLOSE VERY CLOSE	0.16 TO 1 FOOT LESS THAN 0.16 FEET	VERY THINLY BEDDED THICKLY LAMINATED	0.03 - 0.16 FEET 0.008 - 0.03 FEET	FIAD - FILLED IMMEDIATELY AFTER DRILLING
- DRY - (D	ATTAIN OPTIMUM MOISTURE	X CME-55	6 CONTINUOUS FLIGHT AUGER	CORE SIZE:]	2230 HIMA BAO I EET	THINLY LAMINATED	< 0.008 FEET	
PI ΔO	STICITY	L CML-39	X 8" HOLLOW AUGERS	вн		IND	URATION		ROADWAY BORING ELEVATIONS WERE TAKEN FROM THE PROJECT TIN FILE 890092_LS_TIN.TIN RECEIVED ON AUGUST 10, 2022.
		CME-550	HARD FACED FINGER BITS		FOR SEDIMENTAR	ROCKS, INDURATION IS THE HAR	DENING OF MATERIAL BY CEMEN	TING, HEAT, PRESSURE, ETC.	
NON PLASTIC	<u>OTTY INDEX (PI)</u> <u>DRY STRENGTH</u> 0-5 VERY LOW		TUNGCARBIDE INSERTS	X -N <u>O</u>			TH FINGER FREES NUMEROUS GF		BRIDGE BORINGS WERE SURVEYED BY SEPIENGINEERING & CONSTRUCTION, INC. USING A SUB CENTIMETER GPS.
SLIGHTLY PLASTIC	6-15 SLIGHT	VANE SHEAR TEST		HAND TOOLS:	FRIABLE		OW BY HAMMER DISINTEGRATES		
	16-25 MEDIUM		X CASING W/ ADVANCER	POST HOLE DIGGER	MUDEBATE		BE SEPARATED FROM SAMPLE	WITH STEEL PROBE;	
	OR MORE HIGH	PORTABLE HOIST	TRICONE STEEL TEETH	X HAND AUGER	PIODERHIE	BREAKS EAS	SILY WHEN HIT WITH HAMMER.		
CO	OLOR	4 m	TRICONE TUNGCARB.	SOUNDING ROD	INDURATED		DIFFICULT TO SEPARATE WITH	STEEL PROBE:	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).		[□	X CORE BIT	VANE SHEAR TEST	INDOMATED	DIFFICULT	TO BREAK WITH HAMMER.		
			=			SHARP HAMN	MER BLOWS REQUIRED TO BREAK	SAMPLE:	
MODIFIERS SUCH AS LIGHT, DARK, STREAKE	ED, ETC. ARE USED TO DESCRIBE APPEARANCE.				FXTRFMFI		EANS ACROSS CRAINS	LL,	DATE: 8-15-1

R02. P10. B WB IE

See Sheet 1A For Index of Sheets See Sheet 1B For Standard Symbology Sheet

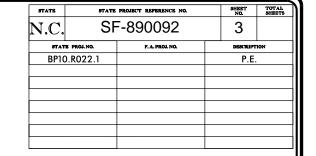


STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

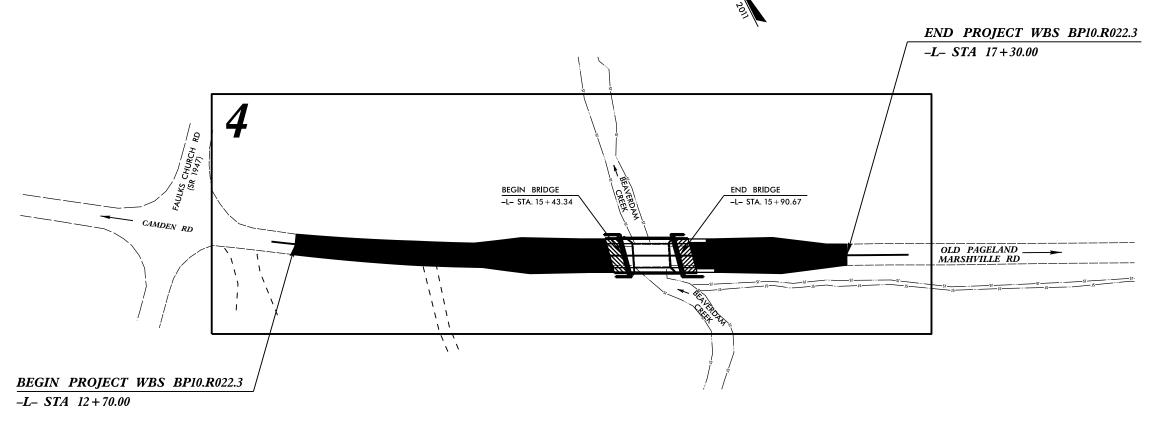
UNION COUNTY

LOCATION: BRIDGE #92 OVER BEAVERDAM CREEK ON SR 1903 (GILBOA RD)

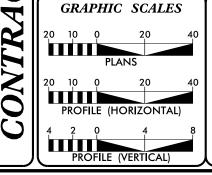
TYPE OF WORK: GRADING, PAVING, DRAINAGE, & STRUCTURE







THIS PROJECT IS NOT WITHIN THE BOUNDARIES OF ANY MUNICIPALITY. CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II. DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



DESIGN DATA

ADT 2019 = 400ADT 2025 = 800DHV = N/A

D = N/AT = 6%V = 55 MPH

PROJECT LENGTH

LENGTH OF ROADWAY PROJECT WBS BP10.R022.3 = 0.078 MILES LENGTH OF STRUCTURE PROJECT WBS BP10.R022.3 = 0.009 MILES TOTAL LENGTH OF PROJECT WBS BP10.R022.3 = 0.087 MILES

> NCDOT CONTACT: GARLAND HAYWOOD, PE Division Bridge Manager

PLANS PREPARED FOR THE NCDOT BY:



RIGHT OF WAY DATE:

JANUARY 18, 2023

NIKKI T. HONEYCUTT, PE PROJECT ENGINEER

STEPHEN L. SAUCIER

ENGINEER SIGNATURE: ROADWAY **DESIGN ENGINEER**



FUNC. CLASSIFICATION: LOCAL SUB REGIONAL TIER

LETTING DATE: **DECEMBER 6, 2023**

HYDRAULICS



October 26, 2022

STATE PROJECT: BP10.R022.1 (SF-890092)

COUNTY: Union

DESCRIPTION: Bridge No. 92 over Beaverdam Creek on SR 1903 (Gilboa Road)

SUBJECT: GEOTECHNICAL REPORT - INVENTORY

PROJECT DESCRIPTION

This project consists of the replacement of Bridge No. 92 over Beaverdam Creek on Sr 1903 (Gilboa Road).

The geotechnical investigation was conducted in September 2022. Standard Penetration Test borings were advanced with a CME 55 drill rig with an automatic hammer. A hand auger was also performed where the use of a drill rig was restricted. Representative soil samples were collected for visual classification in the field. No laboratory testing was performed for this project.

The following alignment, totaling 0.087 mile, was investigated. A plan sheet and cross sections of this alignment are included in this report.

<u>LINE</u> <u>STATIONS</u>

AREAS OF SPECIAL GEOTECHNICAL INTEREST

1. <u>Non-Crystalline Rock:</u> The following areas exhibit shallow non-crystalline rock within 6 feet of the proposed ditch grades:

12+70 to 17+30

<u>LINE</u>	<u>STATIONS</u>	<u>OFFSET</u>	
-L-	14+20 to 15+25	LT	
-L-	14+70 to 15+50	RT	
-L-	16+09 to 17+30	RT	

PHYSIOGRAPHY AND GEOLOGY

The project is located in the Piedmont Physiographic Province. The project corridor is comprised primarily of residential and agricultural properties. The general topography along the project is flat to gently sloping.

Geologically, the project is located within the Carolina Slate Belt which typically consists of metamorphic rocks. Specifically, the formation mapped within the Carolina Slate Belt at the site consists of Metamudstone and Meta-Argillite (CZmd).

Surface water is drained from the corridor by the existing roadway ditches.

SOIL PROPERTIES

Soils encountered during this investigation are separated into two categories based on origin. They consist of roadway embankment and residual soil.

Roadway embankment is present along some of the existing roadway on the project. The roadway embankment encountered generally consist of dry, medium stiff to very stiff, slightly plastic, clayey silts (A-5) and fine to coarse sandy silts (A-4) with trace gravel.

The onsite residual soils are the product of the in-place chemical and mechanical weathering of the parent bedrock, and oftentimes maintain the same layering and lineation of the parent bedrock. Residual soils are derived from the weathering of underlying metamorphic rock. The residual soil encountered consist of dry, stiff to hard, slightly plastic, fine sandy silts (A-4) and dry, medium dense, non plastic, silty, fine sands (A-2-4).

ROCK PROPERTIES

Weathered rock was encountered along the existing roadway (-L-) at elevations ranging from 489.3 to 506.9 feet (MSL). The weathered rock consists of metamudstone and metasandstone.

Non-Crystalline bedrock was encountered along the existing roadway (-L -) at elevations ranging from 488.3 to 496.7 feet (MSL). The non-crystalline rock consists of metamudstone.

GROUNDWATER

Groundwater was not encountered to the boring termination depths at any of the performed boring locations.

Prepared by,

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